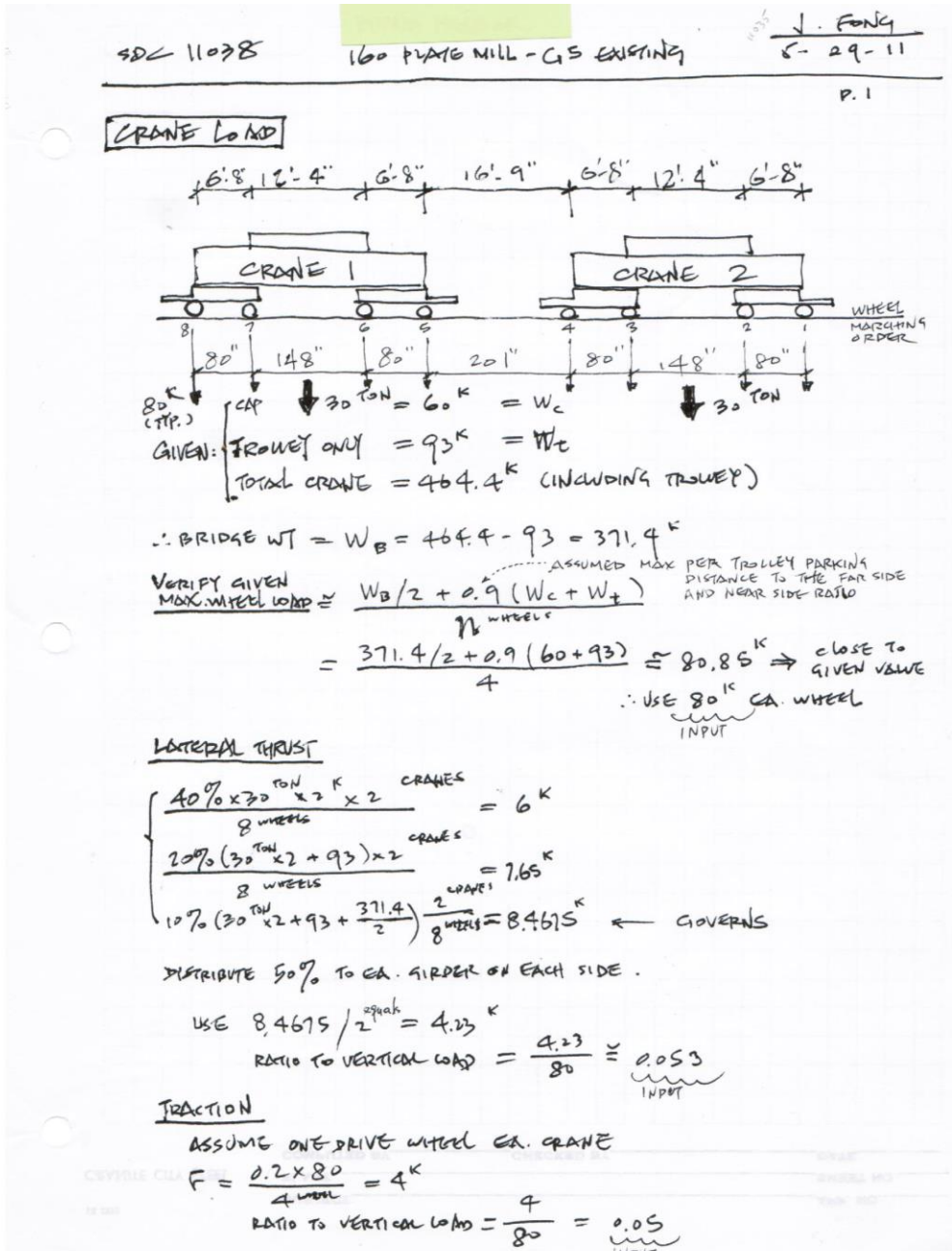


ANALYSIS SUMMARY CALCULATION

The following summary calculation was generated from the results of a CRANE GIRDER PRO structural analysis. The crane girder failed due to the type of cracks shown in Photograph #4 of the section on Crane Girder Failure. The analysis resulted in the replacement of over 10 crane runway girders in length of 50 ft. and 75 ft.

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ASCE EFFECTIVE WIDTH

$$h = 60 + (1.75 + 1.5) / 2 = 61.625$$

$$t_w = 0.5$$

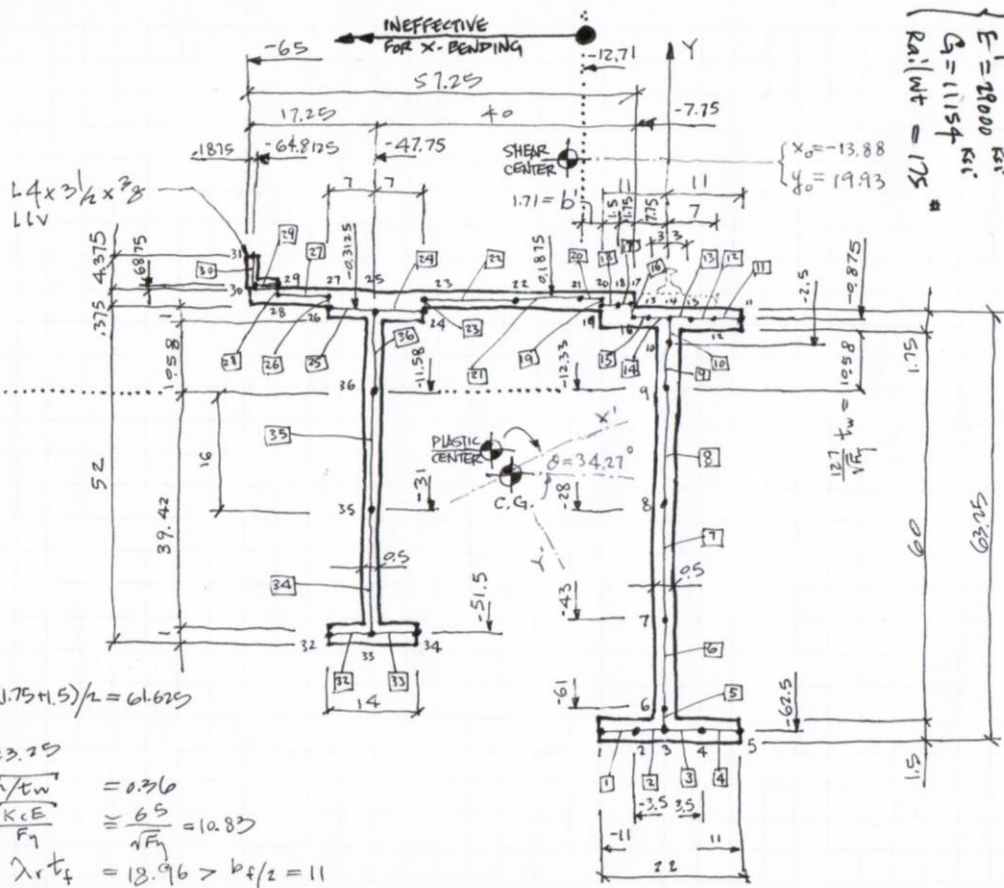
$$h/t_w = 123.25$$

$$K_c = 4 \sqrt{A_g/t_w} = 0.36$$

$$\lambda_r = 0.64 \sqrt{\frac{K_c E}{F_y}} = \frac{65}{\sqrt{F_y}} = 10.87$$

$$b_f = \max \lambda_r t_f = 18.96 > b_f/2 = 11$$

$$b_c = 0.375 \left(\frac{65}{\sqrt{F_y}} - \frac{11}{1.75} \right) = 1.71$$



STRUCTURAL MODEL

$$F_y = 36 \text{ ksi}$$

$$E = 29,000 \text{ ksi}$$

$$G = 11,154 \text{ ksi}$$

$$R_{eff} = 175$$

SPC 11028

160 PLYS MILL - QS AXIS

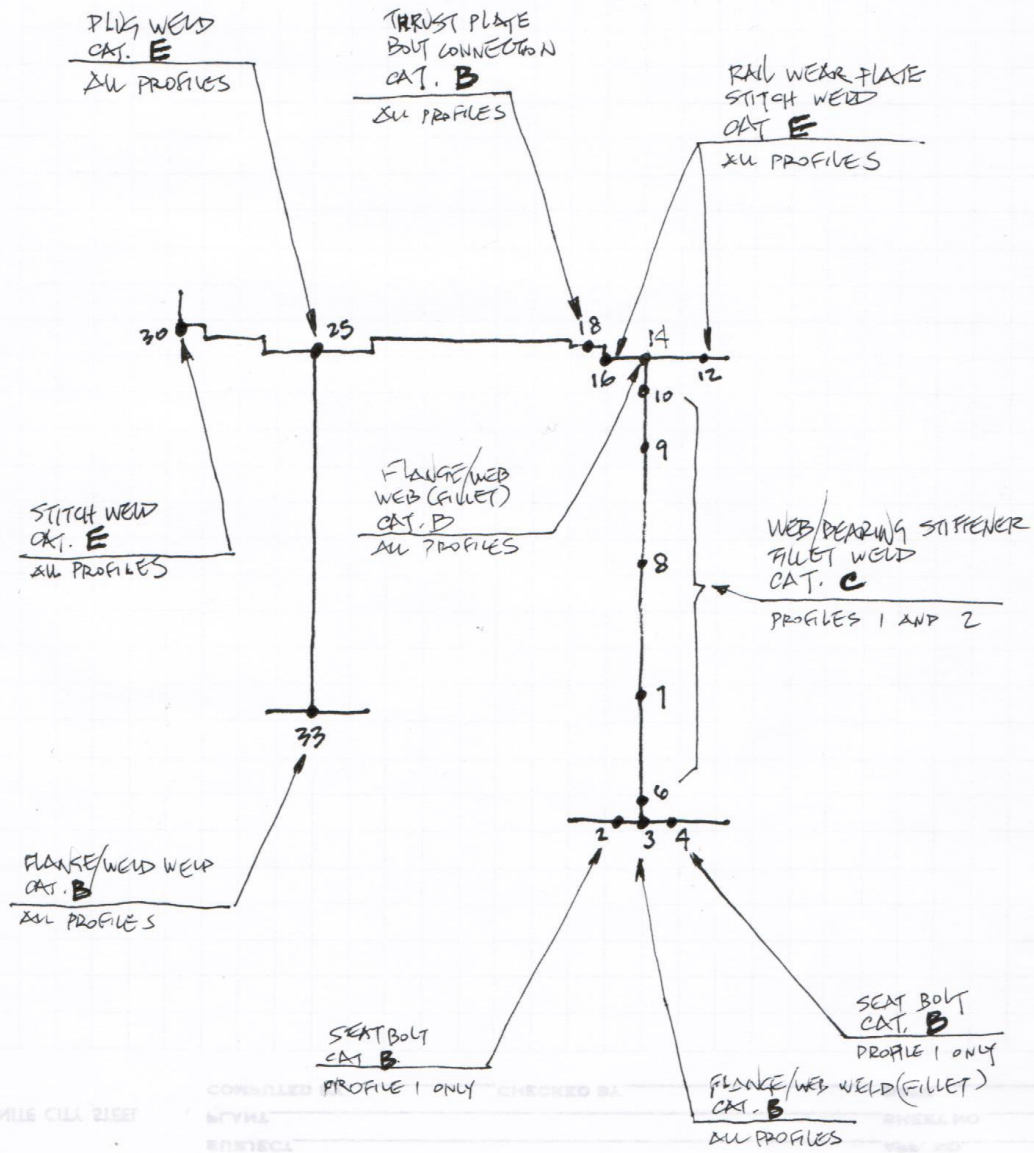
P.2

J. FONG
 5-28-11

TYPICAL PROFILE SETUP

PROFILE FATIGUE CATEGORY

PROFILE TYPE	$\left. \begin{matrix} 1 \\ 2 \\ 3 \end{matrix} \right\}$	APPLIES TO	$\left\{ \begin{matrix} GIRDER END W/ BEARING STIFFENERS \\ \text{INTERMEDIATE SECTION W/ WEB STIFFENERS} \\ \text{INTERMEDIATE SECTION STIFFENER FRGE} \end{matrix} \right.$
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SDC 11038

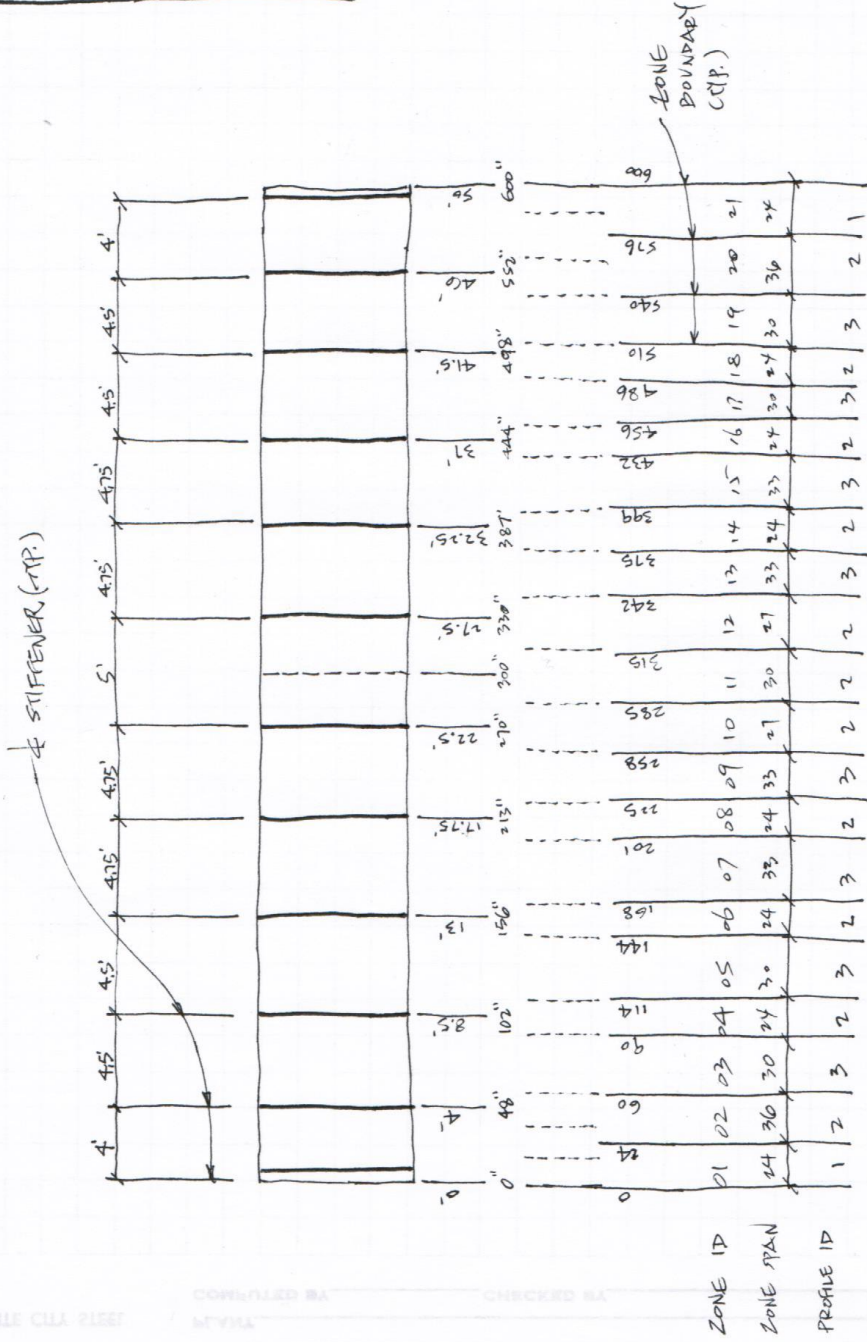
160 PLATE MILL. G5 EXISTING

J. FONG

5-30-11

GIRDER ZONE DEFINITION

P. 4



ZONE II RESULT OVERVIEW

COMPUTED BY _____ CHECKED BY _____

DATE _____

CHAMITE CITY STEEL

SHEET NO. _____

PLANT _____

SUBJECT _____

DATE _____

APP. NO. _____

SPC 11078

160 PLATE MILL - GS EXISTING

J. FONG

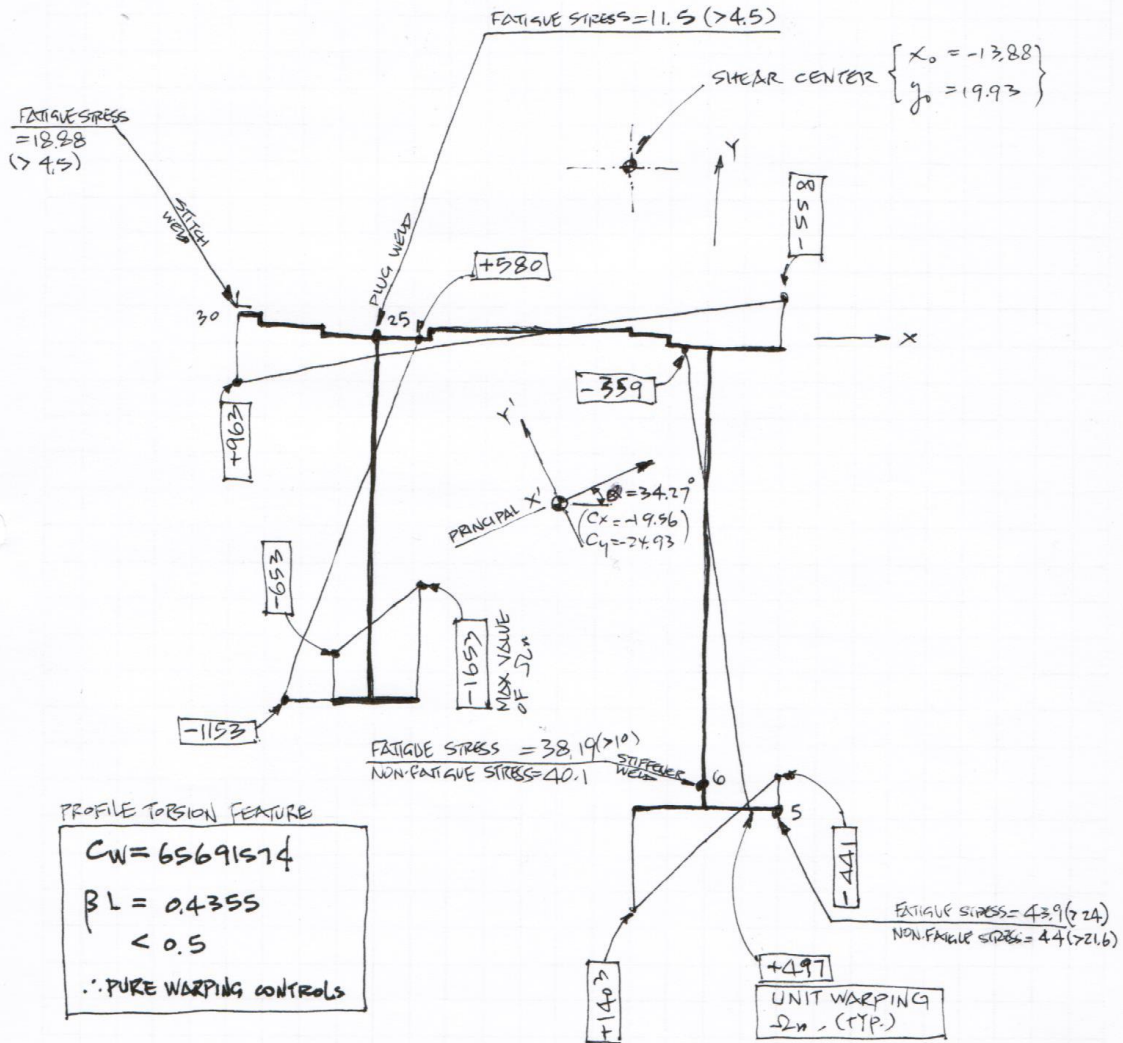
6-12-11

EXISTING GS MODEL ANALYSIS

P. 5

RUN ID GSEXFLD4 (FOR FATIGUE LOAD CONDITION 4)

GSEXFLD3 (FOR FATIGUE LOAD CONDITION 3)



ZONE II RESULT OVERVIEW

COMPILED BY	CHECKED BY	DATE
CRYMILE CUI ZIEF		
TRIAL		
SUBJECT		

EXISTING GIRDER EVALUATION COMMENTS GSEXFLD 4

1. GIRDER PROFILE RESEMBLES AN UPSIDE-DOWN U OR A LAY-DOWN CHANNEL SECTION, FOR WHICH THE SHEAR CENTER IS WAY ABOVE THE CHANNEL WEB. THEREFORE THE ENTIRE GIRDER SECTION IS PREDISPOSED TO ROTATE ABOUT THE SHEAR CENTER WHEN LOADED AT TOP OF THE RAIL FROM THAT THE RESULTANT "RARELY" PASSES THROUGH THE SHEAR CENTER.
2. TORSION CHARACTERISTIC CONSTANT $\beta = 0.0007258$, LENGTH $L = 600$. $\beta L = 0.4355$. ANY SUCH βL MAGNITUDE, LESS THAN 0.5, WOULD CLEARLY INDICATE THAT ST. VENANT TORSION (COMPARING WITH WARPING) IS RELATIVELY MUCH TOO SMALL TO HAVE PRONOUNCED EFFECT. THEREFORE THE GIRDER'S TORSIONAL BEHAVIOR WOULD BE CONTROLLED BY "PURE WARPING".
3. FIBER STRESS AT NODES IN THE BOTTOM FLANGE AND THE LOCALITY IS GENERALLY HIGHER THAN THAT ELSEWHERE. THIS IS TRUE FOR ALL ZONES DUE TO THE FACT THAT THE DISTANCE TO THE CENTROID AND/OR SHEAR CENTER FROM THESE NODES ARE INHERENTLY FARTHER THAN THAT FROM OTHER NODES.
4. EXCEPT FOR THE END ZONES (1, 2, 20, 21) UP TO ABOUT 5 FT FROM THE SUPPORTS, THE GIRDER IS FOUND OVERSTRESSED UNDER NON-FATIGUE LOAD AT VARIOUS HOT SPOTS IN ALL OTHER ZONES. TYPICALLY, HOT SPOT INCREASES IN NUMBER FROM MINIMA IN ZONE 3/19 TO THE MAXIMA TOWARDS CENTER ZONE 11.
5. SUBJECTED TO REPEATED STRESS FLUCTUATIONS, THE BASE METAL SHOULD DEVELOP CRACKS AT MOST HOT SPOTS ESPECIALLY IN ZONE 11. LISTED AS FOLLOWS ARE SOME OF THE HIGHEST RATIO OF BASE METAL FATIGUE STRESS (REVERSAL) TO DISC ALLOWABLE FSR VALUES UNDER FATIGUE LOAD CONDITION 4 ARE:

4.2 (= 18.8 / 4.5)	}	AT NODE 30 - KICK ANGLE / # STITCH WELD
3.82 (= 38.19 / 10)		6 - WEB STIFFENER BASE WELD
2.56 (= 11.5 / 4.5)		25 - SUPPORT BEAM / # PLUG WELD
1.83 (= 43.9 / 24)		AT NODE 5 - BOTTOM FLANGE TIP (NO WELD)